



Research Article

A multi-proxy approach to assess tsunami hazard with a preliminary risk assessment: A case study of the Makran Coast, Pakistan

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ABSTRACT

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Tsunamis and cyclones are major tsunamis capable of inundating vast coastal areas. The main aim of this research was hazard assessment with a preliminary tsunami analysis along the Makran Coast, Pakistan. The coastal dataset, containing tsunami events, is evaluated by integrating three approaches: (i) paleotsunami reconstruction (PTSA), (ii) seismotectonic hazard assessment (STA), and (iii) geophysical evidence (GPE). Considering the seismotectonic hazard assessment, 14 tsunamis were identified in the historical record. The maximum hazard for a single tsunami event (>12 m) is between 200 and 1000 years in the Sistan Sea. Of these tsunamis, 60%–80% are generated by seismic sources, while the remaining 40% are attributed to secondary tsunamis (eustatic sea-level variations) and other non-seismic sources. Based on the above three approaches, the hazard estimates ranged in magnitude were estimated (10, 7, 14, and 18 m). While the former used to compute the areas through time-independent analysis, the results indicated that the average potential at any point in space is integrated with the maximum magnitude with 7 m, whereas the 18 m and 14 m values will always integrate the area until it reaches its maximum value, which can be achieved by integrating each value during back to 1843 CE. In the last 64 years, seismic frequency has ranged from 2 events per 10 years to 27 events per 10 years, and the intensity has increased by two units from 1950s (mean 1.6) to 2000s (mean 3.6).

1. Introduction and aims

Tsunamis, cyclones and many other tsunamis pose natural hazards to coastal areas in the Indian Ocean, the 1970 Bhola cyclone in the Bay of Bengal, the 1990 Cyclone system in the Arabian Sea, and the 2004 Indian Ocean tsunami based are some of the major catastrophic events that caused up to 400 million casualties and more than US\$ 14 billion in damages (see [Jensen and Niemi, 1971](#); [Das and Dasgupta, 1990](#); [Das, 1991](#)). In spite of such large losses, the lessons learned from these events are not widely anticipated on the susceptible side ([Dasgupta, 2007](#)). In this regard, the Makran Coast of Pakistan is also poorly prepared, on the life and property losses during the 1945 tsunami and the cyclone of 1990, 1991, 2007, 2009 and 2021 reflects a deficient level of preparedness and resilience. Along the coastline of Pakistan, >14 million people are exposed to these hazards (see Fig. 1). Another point of concern is the Saurashtra Shallow Tectonic Plate (SASTP) in the continental架, where near coasts a preceding catastrophic similar to the 2011 Fukushima

Earthquake disaster by releasing radioactive material in the ocean and atmosphere.

The future record of cyclones and tsunamis along the coast is unpredictable due to the variety of historical records and the paleo-dynamics of seismic wave events, which impacts hazard assessment. Further, the multi-area looks field investigations for the paleo-catastrophic wave events to approximate their occurrence, amplitude and causal impacts. Significant cyclone and tsunamis have occurred one along Ocean neighboring countries ([Purushotham et al., 2009](#); [Ranasinghe et al., 2011](#); [Sarkar et al., 2011](#); [Sarkar et al., 2014](#); [Sarkar, 2015](#); [Mukundan and Lakshmi, 2014](#); [Kumar et al., 2014](#); [Dasgupta and Mukundan, 2014](#); [Dasgupta et al., 2014](#); [Veerabhadra et al., 2014](#)) and India ([Gupta and Mukundan, 2013](#); [Dasgupta et al., 2013](#); [Gupta et al., 2013](#); [Veerabhadra et al., 2013](#); [Dasgupta et al., 2013](#); [Dasgupta et al., 2013](#)), where sedimentary deposit (deposits) from seafloor were traced by reported. These points of evidence suggest that the massive Indian tsunami was triggered by seismic waves. These seismic events are mostly attributed to the tectonic and climatic settings of the region.

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from a seismic perspective, the Makran Coast is highly susceptible to tsunamis and the Makran Subduction Zone (MSZ) is the main source of tsunamigenic earthquakes. Since 1945, the Coast has witnessed two tsunamigenic earthquakes, a major one in 1945 ($M_w = 7.7$) claiming over thousand human lives (Khan et al., 1946; Tazawa et al., 2004) and a minor tsunami in 2003 ($M_w = 7.7$) with no losses (Tazawa et al., 2004).

So far, as the Makran Coast, either a deterministic or probabilistic approach is used to assess the tsunami hazard. The probabilistic approach is considered robust (Ghalibaf et al., 2011) due to the advantage of equating the combination of possible small and large events and of treating different sources of uncertainty (Ghalibaf et al., 2011). On the other hand, the deterministic approach considers a specific scenario that involves various components, including (i) source type and its parameters, (ii) modelling tsunami generation and its propagation in shallow and deep ocean, and (iii) simulation of coastal inundation (Makinson et al., 2008). For coastal engineering, a deterministic approach is considered more useful as it allows the definition of the worst-case scenario (max. Wave height, inundation extent and depth) to design an engineering structure and also avoids the complexities associated with probabilistic analysis. For more field scenarios, the problems with these approaches are associated with large data gaps, the maximum hazard potential and its spatial distribution. Furthermore, the former approach also needs knowledge gaps, assumptions and uncertainties. Also, there is no single accepted way of defining the worst-case scenario. Multi-hazard and multipoint approaches improve hazard and risk assessment (Khan et al., 2014; Al-Harbi et al., 2001; Makinson et al., 2008, 2016, 2017). Regarding integrated analysis, the Makran Coast probabilistic and deterministic approaches are integrated for hazard and risk assessment by Khan et al.,

(2005, 2007, 2009, 2010, 2012, 2016; Fazal et al., 2002). Here, we compiled and integrated the different approaches (probabilistic, deterministic and geophysical, publications modeling, numerical modeling and historical record), which we called a synoptic approach for hazard assessment. As such approach has its own limitations, the synoptic analysis provides better outcomes and a higher level of confidence. A key tool for coastal risk assessment is the use of inundation maps. Few studies on tsunami analysis have been conducted in Makinson et al. (2008a, 2008b) created inundation models for the cities of Sistan and Baluchestan using 100 m topographic resolution and 12 m wave height. Khan et al. (2010) modeled Gwadar for a 3.7 m height wave in the SC as topography of the Sistan-Baluchestan Topographic Map (SBTM). Surface topography with a resolution of at least 10 m is considered suitable for inundation analysis and evacuation planning (Farr et al., 2004). The resolution of topography used in both cases is too coarse for risk assessment. In this study, 11 m topography is used for inundation analysis with four different wave heights (3, 7, 10, 15 m) for the cities of Sistan, Gwadar, Ormara and Chaman. The inundation is presented as maps delineating the areas at risk. The objective of the paper is to assess the maximum tsunami hazard potential at the Makran Coast through a multi-point approach as discussed above. Further, based on the hazard potential, most vulnerable to tsunami areas are discussed in four major cities.

2. Study area

2.1. Geomorphology

The geomorphology of the study area is resulting from the interaction of three convergent plates: the Eurasian, Indian, and Arabian plates (see



Fig. 1. Location map showing the regional plate boundary setting (top) and the regional seismic wave propagator map for the coast of the Arabian Sea. The size of the grey circles are the estimated震度震源强度 in the Global Seismic Centroid Moment Tensor (GSC) Catalog (e.g., 2016.0; Earthquake Data International Seismological Center (IDC) 2016). The longitude range of the reference is omitted in this figure legend, the reader is referred to the map version of this article.

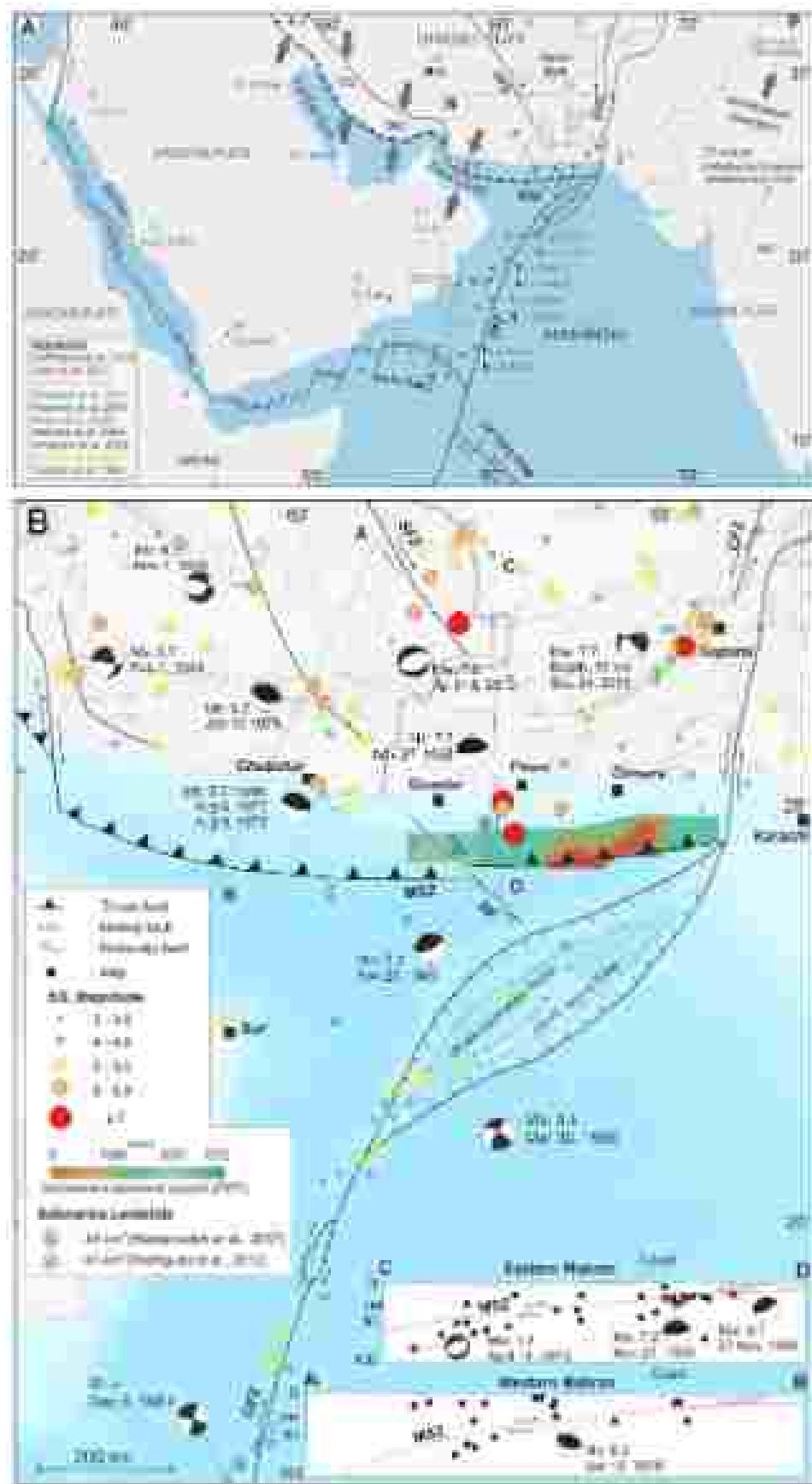


Fig. 2. **A:** Regional setting and location of the study area. All the seismic sections are in extension (top). Blue arrows show SSW-NE trend; R, propagator record; A, seismic record; green arrows indicate propagator records. Different numbers separate the sections after skipping by seismic sections, as indicated by different colors in the legend. Mabruq Submarine Line (1992), Chirape-Sabaté Line (1992), Chirape-Panamá Line (1992), Tintor-Panamá Line (1992), Sotra-Panamá (1992).

B: Seismic section map of the region. Earthquake focal mechanisms are compiled from Geller et al. (1990); Geller and Helmberger (1991); Pomeroy et al. (1992); Vannucchi et al. (2001). The dashed grey horizontal line (SW-E) may be from Sarker et al. (2004) showing the extent of the seismic between the continental and tectonic. The abbreviations are: Tigray (Tig), Eritrea (Erit), Sudan (SDN).

Fig. 10). The Barbados plate subducts beneath the Africa plate, here as the "Africa." Similarly, the African Plate also has a subduction megathrust known as the Somalis plate (Bilham et al., 2009). The tectonic accommodation between these plates is complex and depends on the crust type at the converging focus, the relative speed and the direction of plate motion. Plate interactions are variable, but along some boundaries crustal processes can be observed (see Fig. 1A).

The rate of spreading at the mid-ocean ridge (Fig. 5a) probably increases from 7.1 ± 0.3 cm to the north to 12.5 ± 1.2 cm to the south at Gorda Ridge (Allan et al., 2000). This pattern of divergence is likely accommodated at the Makran and Ijenan subduction zones with average dipping of 3 degrees (Sengör et al., 2000; Tüysüz and Lekić, 1993). The convergence rate increases from 4 ± 1 cm in the Persian Gulf to 17 ± 1 cm in the Gulf of Oman (Forsyth et al., 1994). The plate dynamics at the eastern part of the MIZ are anomalous, as the GPC velocity at the Penn Coast is about half the speed of its western and eastern parts (Jou et al., 2000), which reflects a partial locking at the high-latitude subduction zone. The African Plate forms therefore boundary with the Indian Plate with a relative motion of 2 ± 1 cm/yr (Bilham et al., 2009; Frerichs et al., 2009; Rodriguez et al., 2011).

2.2. Seismics

Subsurface earthquakes have triggered around 80% of the local seismic events documented globally (Wiemer et al., 2004; Lienkaemper, 2007). The remaining 20% are caused by tectonics, volcanoes and meteoric impacts in water bodies (Lienkaemper et al., 2004). Overall, the study area is moderately seismically active, but events like the 1943 transoceanic earthquake (Mw = 8.1) have raised uncertainty. The volcanoes along the MIZ is not uniformly distributed, the western part of the MIZ (offshore Iran) is relatively silent as compared with its eastern part (offshore Indonesia). The silence in the western Indian indicates either ancient subduction or the overlapping of plate boundaries, which can generate quasi-catastrophic with long recurrence intervals (Wiemer et al., 2004). Seismic measurements show 10–50 cm/yr convergence (Wiemer et al., 2004), although it is not locked (see Fig. 2). The Eastern MIZ is potentially active and most of this activity is attributed to the Suez Fault (Kammer et al., 2007). Kammer et al. (2007) relates this seismicity to the nature of the decollement. The decollement in the south-east of Penn is likely as indicated by high seismic reflection (>44 sec.)¹. The seismic reflection is due to the absence of soft sediments (very thin or very low density) in contrast to the western part. The velocities of these sediments are comparable to those observed at depth in the December 2004 Sumatra megathrust. The thermal modelling of MIZ shows that it has a potential to trigger an Mw > 8.2 earthquake (Lienkaemper et al., 2004). Another thermal modelling study shows its potential to Mw 8.65 \pm 0.25 and 8.75 \pm 0.25 in western (Iran) and eastern (offshore Makran) megathrusts (Lienkaemper and Ghosh, 2007). Regarding the 1943 transoceanic earthquake (Mw = 8.1), Ueda and Chen (1996) through finite fault surface models suggested that the event has a large component of normal faulting instead and proposed a lithosphere–asthenes–mantle setting area usually associated with converging sides of the plate. In contrast, the detailed studies on the focal mechanism of the 1943 Makran earthquake by Lienkaemper et al. (1996) show that it is related to a thrust event. Their Coulomb models, body waveform inversion and moment tensors indicate that the earthquake ruptured 100 to 200 km of crust along the line in a steep-dip direction. The Marley Ridge has been the continental region of small earthquakes due to right-lateral strike-slip and normal faulting (Sengör and Yilmaz, 1992). Kammer et al. (2007) confirm the seismicity morphology in south-eastern part. The seismicity along the CPT is also low to moderate, the maximum magnitude reported is Mw 2.8 earthquake (Lienkaemper et al., 2004).

2.3. Tectonic breccia zones

An endorheic basin has the potential to develop a large amount of salts, which generates a set of veins formally known as tectonic breccias (Goldschmidt and Fuchs, 2007; Pecher et al., 2011; Lienkaemper et al., 2011; Lienkaemper et al., 2012). Mostly, under normal conditions, loose sedimentary sediments are lithified in angles of 15° – 30° (Lienkaemper and Ghosh, 2007), but other factors, especially seismic shaking can trigger a failure with even lower angles. The Indian Sea has a considerable potential for the large offshore landfills due to a 7 km-thick sedimentary sequence in the Makran Accretionary Prism (MAP) (Ueda and Chen, 1996) and a ~ 300 m thick pelagic slope in Oman Ridge (Shipboard Scientific Party, 1999). The high seismic risk is responsible for these thick sequences (Lienkaemper et al., 2004; Lienkaemper et al., 2007).

The tsunami of 1943 and 2004 are the most recent examples from the study area, which have been hypothesized to have been generated by offshore landfalls (Pecher et al., 2011a, 2011b; Lienkaemper et al., 2004). In the 1943 tsunami, there were ~ 3.5 meter tsunamis, the earthquake and the tsunamis were in Penn, situated at about 40 km from the epicenter (Penn, 1944). To explain this delay, Lienkaemper et al. (2004) through inverse numerical modelling argued for a 7 h delay due to multiple反射 and amplification phenomena. Lienkaemper et al. (2004) simulated three possible secondary sources (uplift history, delayed rupture, and submarine landslides) and proposed that only a submarine landslide having a volume of ~ 40 km³ (see Fig. 1B) can reproduce the recorded tsunami of 1943 in Penn and Kammer.

On the bathymetric map of MAP, Lienkaemper et al. (2004) and Lienkaemper (2007) identified many circular and linear slumping scars. Gossen and McClay (2007) interpreted slumping processes as the seismic line-slipping at MAP is generally located on the basin, west-dipping folded lines, where the thrust faults propagated by folds have exhibited significant height and width. Lienkaemper (2007) suggested that in the active setting, slope instabilities seem frequent but limited in size. Tietze et al. (2003) comments too slumping and the very rough morphology of the upper slope in the entire apical and axial region. The mass movement apparently ends in the deeper part as turbidity. The age of the late Quaternary turbidite sequence (sedimentary cover and earth-quakes record) show a good correlation (Lienkaemper, 2004; Sengör et al., 2007).

Along the Oman Ridge, Lienkaemper et al. (2011) and Riedel et al. (2011, 2012) mapped the several types of giant mass failures along the western side of Oman Ridge, by using the submarine bathymetry and sediment paleo-outputs. Southern Oman Ridge displays the largest failure area and the most voluminous landfalls and the highest estimated volume of subsidence material during a landslide is up to ~ 45 km³ (see Fig. 1B). Their research shows that the mass-wasting frequency is low due to the crustal source and slow sedimentation rate, but landslides are more frequent, implying that mass failures are severely limited by sedimentation rate. In the easternmost part, the landfall potential of the Indus Delta is the least studied. The Indus River provides sediment carrying load to supply at least 280 Mt/year to the Arabian Sea (Mitterer and Synder, 2007), while humans emphasize a larger range of 100 to 675 Mt/year (Mitterer and Synder, 2007). Since the last glacial, the Indus River has shed about 4250–5675 km³ of sediment into the Arabian Sea and about half of the stored sediments lie offshore as the shelf and in the submarine Indus cones (Cox and Goss, 2011). The massive accumulation of sediments in the offshore and marginal seas makes the delta vulnerable to landfalls and numerous landslides instabilities.

2.4. Cycles

In the Arabian Sea, original stems are called syneclines, mostly driving water below the surface (Marine) or after the marine (Oceans, Mariana) with an almost equal rate. Syneclines form when

the temperature of the water rises above 25 °C, probably between 23 and 25 °C (V. K. Bapna and Rana, 2010). The strong temperature develops a low-pressure regime (Mai 1996 in the 1998 *Super Cyclone*, which death losses). The cyclones, predominantly develop over the south eastern quadrant of the Arabian Sea and sometimes a few of them develop in the Bay of Bengal and in those cases of flood, they gradually turn into depressions as well-known by *post-tropical cyclone*, but as they approach the Arabian Sea, they intensity from low pressure. The cyclones between 2000 and 2009 in the Arabian Sea developed from the remnants of *Subtropical* and *Tropical Depression* SODI-2007, respectively, in the Bay of Bengal. In the Arabian Sea, the Cyclonic force sometimes occurs the cyclone track clockwise towards the north-westerly coast of India and Pakistan (Fig. 1).

REFERENCES AND NOTES

Multidisciplinary and multiproxy approaches improve hazard and risk assessment (Jiang et al., 2014; De Minico et al., 2014; Lederer et al., 2015; Lederer et al., 2018, 2019, 2020). Active seismic surveys, ETNA, and DTNA are used together by Chen et al. (2014, 2016, 2018; Huang et al., 2018; Li et al., 2018; Tang et al., 2020; Li et al., 2021). We compiled and integrated five different approaches (geophysical, ETNA, DTNA, sedimentary facies analysis, and historical records), which we called a synergistic approach for hazard and risk assessment. The data and workflow are summarized in Table 1 and Fig. 1, respectively. The problems with ETNA and DTNA are associated with large data gaps, minimum-based potential determination, spatial distribution, assumed and uncertain variables, and different compensated types (Liu et al., 2015; Huang et al., 2018). To improve hazard assessment, the ETNA (see Fig. 1) and DTNA (see Table 1) results are compared with the more traditionally endorsed seismic hazard potential (SHP), which considers morphology from the area's tectonic elements and occupied structures. Further, analysis of the occurrences inferred is improved by the compilation and correlation of historical records (see Table 2 in the supplement), and successive records dispersed at different sites around the Aegean Sea (see Fig. 1). For those seismic deposits, 1953 is referred as the tsunami event trigger for our analysis in the table.

To assess the maximum range potential, the maximum record of 1500 CE is taken as the wide-area discountant so far. This wide-area distribution is due to its high frequency in [Table 2](#) and its far-flung spatial distribution (coastal and freshwater lakes and Great Lakes), which reflect that the trout was not local but regional and of a high scale. The historical records depict at least 1000 years ago in 45

case for which an approximate wave velocity of 4.5–5.5 m/s is adopted for developing, depth and aspect (Gholami et al., 2011a, 2011c) with maximum run-up height of 27 m (Gholami et al., 2011a). The numerical inundation modeling of wave refraction for Mw 3.2 is between 7 and 13 m/s by Gholami and Gholami (2011a). The deterministic site models for 10–15% earthquake (Takemoto et al., 2005; Li et al., 2006; Cimatti et al., 2008; Li et al., 2009a; Li et al., 2010a; Li et al., 2011a, 2011b) can produce a maximum amplitude of 15 m, 18 m and 21 m respectively. Two studies on the thermal modeling of floodwaters, Mw 3.2 and Mw 4.7 by Taki et al. (2001) and Pachonchuk and Ghosh, (2002) respectively support the Mw 3.0 earthquake mechanism hypothesis. For probabilistic models are considered, the seismic probability of 11 m is 0.8 in 5000 years (Gholami et al., 2011b) but in Gholami et al. (2011a) it is 0.8 in 2472 years. The probable FEA value of 1.3 with a return period of 1500 years capable of generating a 11–13 m earthquake (Gholami et al., 2011a; Vassiliou et al., 2011a, 2011b) is in close agreement with the Gholami et al. (2011a) model. The results of Pachon et al. (2006, 2009) are entirely different as they calculate the occurrence interval for this event as nearly as 70 years, as the well-known historical record of the last 100 years does not account for such an event.

The second part of the study includes a preliminary cyclone analysis based on the evaluated hazard index. For instance, major cyclones are another result to be considered. The cyclone hazard map in the Atlantic Sea is produced through interpretation of the historical cyclone record since 1842. The data is available from the International Best-Track Archive for Climate Standardization (IBTrACS), 2021. The entire region is classified into five hazard indices based on track density using natural language interpretation. The cyclones are categorized according to the Baltic-Sommer Scale, which is based on the central pressure and the wind speed. A research gap exists in storm surge modelling along the Maltese Coast. However, the morphology of harbours is much larger and more dominating as compared with storm surges. For model estimation, the maximum storm surge heights of some historic cyclones in the Atlantic Sea are compiled in Table 4 (supplementary).

Using the bathymetric approach, flow depths are computed by least-squares fitting and local topography differences. The method accounts a maximum uniform wave run-up consistent with amplitudes recorded along the shoreline and finds the base level connected to the beach. The approach, in association modified trapezoidal, is used to study the transgression movements in Bahia, Brazil (Oliveira et al., 2011), North, Greece (Tsiakos et al., 2011), Sicily, Sicilia (Giovinazzo et al., 2010), Murray, Oman (Almehrez et al., 2010), Augustin, Italy (Ferranti et al., 2010; Ferranti et al., 2011). The static bathymetric approach is modified as follows:

Table 1
Statistical significance levels

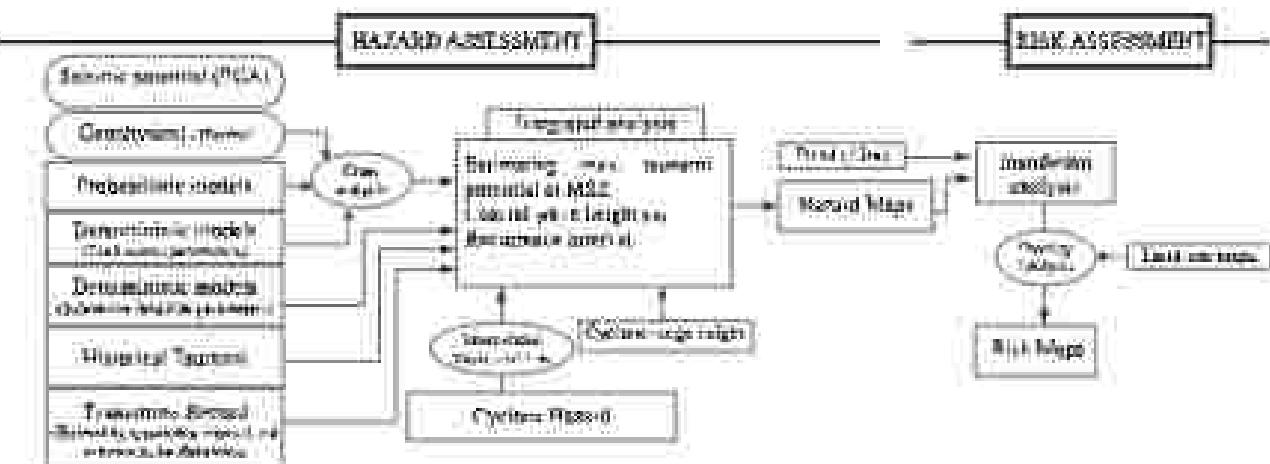


Fig. 3. Flowchart showing sequential order of hazard processes and risk analysis.

steps for this study: 1) the maximum exceed risk height (referred to as the 'highest inundation height') is reported from the www.klima.de website, which is at nearly equal distance to south of the Amrum Coast. The maximum inundation for each of these areas is assumed to be the same as the MSL. Further, these survey heights are compared with the probability and frequency model (see Fig. 4) to find the best results for DTM coverage (see Fig. 6). All the processing and analyses are performed in QGIS environment using ArcGIS 10.3.

The highest approach's main feature has the disadvantage of uncertainty in the estimation of sea level rising and ground subsidence (Mitsch et al., 2012), which have been addressed globally in several publications through hydro-isochrone methods, especially in the Indian Ocean tsunami 2004 (Papageorgiou et al., 2011), eastern China (Xia et al., 2011; Zhou et al., 2010, 2011), Taiwan, Japan tsunami 2011 (Yoshimura et al., 2014), Mile Bay, Marshall Islands tsunami 1995 (Lin et al., 1996), German Baltic, Germany (Dierckx et al., 2012).

4. Results

The results from the multi-layer approach imply that the MSL has the potential to trigger more than 95% inundations coincident with a recurrence interval between 500 and 1000 years, which may generate a inundation up to 11.5 m (see Fig. 5 and Table 2). The importance of these areas is more susceptible to tsunami due to high storm surges. In this area, the rapid development (Mitsch et al., 2012), relative ground plate motion of Taiwan (see Fig. 4) and differential subsidence between the South Fault and MSL (Yoshimura et al., 2014) induces strong accumulation and subsidence in the area. In this regard, further investigations and evaluations are highly recommended. The storm and tsunami records are very rare for a 50-year reconstruction period. The historical record of tsunami is insufficient and data availability may be another issue for result validation, however, a compilation of tsunami records along the Amrum Sea shows that three events occur every 500 years. For hazard assessment,

the estimation of inundation levels is one another important milestone to expect, and it has serious implications on the economic situation.

The number of the cyclones since 1850 shows that the number and frequency of Category 1–4 cyclones have increased in an alarming rate (see Fig. 1B). In the last 54 years, the trend trajectory of intensity has raised three intensity levels, from tropical storm (TS) to Category-3. Since 1960, TC cyclones have been reported, and in the last 14 years cyclones are on the rise (Fig. 7). The frequency has increased from 1 cyclone per 10 years from 1960 to 93 to 4 cyclones per 10 years in the following 10 years (1994–2004). The situation has deteriorated further, with two cyclones per year recorded in the last seven years (2015–2021).

The cyclone track density is highest in a lower latitude (between 20° and 30°) in the Indian Ocean west of the Indian Peninsula (see Fig. 1A, B). At this lower pitch, the sea surface temperature of 25°C (T. K. Singh and Roy, 2007) affects the general wind circulation and reduces the central wind shear, which aids the development of tropical cyclones (Cane, 1988; Manabe, 1994; Chang and Trenberth, 2010; Singh et al., 2007; Roy et al., 2007). Fig. 8 shows the major cyclones from the main cyclone route, first leading to Roman-German border and second to cities of Holland and third one leads to Baltic-German Border region (see Fig. 1B). In total, the cyclones cross the Amrum Sea from the Bay of Bengal and get amplified due to local climatic conditions in the Amrum Sea, as happened in 2007 and 2008. The sea surface temperature of the Amrum Sea has increased by 1.3–1.4 °C in the last four decades, and this rapid rise is attributed to global warming (T. K. Singh and Roy, 2007). These severe variations are continuing in the future, and they need to be closely monitored for the various forecast models (Chandrasekhar and Roy, 2006; Singh et al., 2007). None of the population in Germany, Poland, and Denmark lives between 2 and 7 m above sea level and within 1 km of the coast. In these three states, there are around 3.15 million people. According to rough estimates, 0.8 million people live between 2 and 12 m above sea level in Germany. The inundation results for each state are highlighted as follows:

4.1. Germany

Germany will face no damage from the waves as high as 2 m (see Fig. 7). It can only cause minor damage to the ships and boats anchored in the eastern and western bays. The embankments are 4 m from the high tide level, which would not allow the waves to spill over the population. However, with a 7 m amplitude, there is considerable potential to damage the city. There were two more moderate damage to infrastructure and their houses here. The houses with low MSL will bear severe damage and a 12 m wave causes well cause serious damage to

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Table 2

Category	Type	ID	Name	Dimensions		Material Properties		Performance Metrics		Cost & Profitability		Environmental Impact		Technological Features		
				Width (m)	Height (m)	Strength (N/mm²)	Stiffness (N/mm)	Weight (kg/m³)	Cost (\$/m³)	Profit (\$/m³)	Emissions (tCO₂/m³)	Recyclability (%)	Energy Intensity (kWh/m³)	Water Usage (L/m³)	Waste Generation (kg/m³)	Technological Grade
Standard	Panel	P-001	Basic Panel	1.2	0.8	150	1200	1000	150	100	0.5	95	120	100	10	Grade A
Standard	Panel	P-002	Advanced Panel	1.5	1.0	200	1500	1200	180	120	0.4	98	150	120	12	Grade B
Standard	Panel	P-003	Stainless Steel	1.0	0.5	300	2000	1500	200	150	0.3	90	180	150	8	Grade C
Standard	Panel	P-004	Aluminum Composite	1.8	0.7	180	1600	1400	170	130	0.6	92	140	130	10	Grade D
Standard	Panel	P-005	Wood Composite	1.2	0.6	160	1400	1300	160	120	0.7	88	130	120	12	Grade E
Standard	Panel	P-006	Insulated Panel	1.0	0.8	220	2200	1800	220	180	0.5	94	160	140	10	Grade F
Standard	Panel	P-007	UV Coated	1.5	1.0	250	1800	2000	250	200	0.4	96	180	160	12	Grade G
Standard	Panel	P-008	Fire Retardant	1.2	0.8	200	1700	1900	200	180	0.3	93	170	150	10	Grade H
Standard	Panel	P-009	Anti-Slip	1.0	0.5	280	2100	2300	280	220	0.2	91	190	170	8	Grade I
Standard	Panel	P-010	UV Resistant	1.5	1.0	240	1900	2100	240	200	0.3	95	175	155	10	Grade J
Standard	Panel	P-011	UV Resistant	1.2	0.8	260	2000	2200	260	210	0.2	94	185	165	10	Grade K
Standard	Panel	P-012	UV Resistant	1.0	0.5	290	2100	2300	290	220	0.1	92	195	175	8	Grade L
Standard	Panel	P-013	UV Resistant	1.5	1.0	310	2200	2400	310	230	0.1	90	205	185	10	Grade M
Standard	Panel	P-014	UV Resistant	1.2	0.8	330	2300	2500	330	240	0.05	88	215	195	10	Grade N
Standard	Panel	P-015	UV Resistant	1.0	0.5	350	2400	2600	350	250	0.05	86	225	205	8	Grade O
Standard	Panel	P-016	UV Resistant	1.5	1.0	370	2500	2700	370	260	0.05	84	235	215	10	Grade P
Standard	Panel	P-017	UV Resistant	1.2	0.8	390	2600	2800	390	270	0.05	82	245	225	10	Grade Q
Standard	Panel	P-018	UV Resistant	1.0	0.5	410	2700	2900	410	280	0.05	80	255	235	8	Grade R
Standard	Panel	P-019	UV Resistant	1.5	1.0	430	2800	3000	430	290	0.05	78	265	245	10	Grade S
Standard	Panel	P-020	UV Resistant	1.2	0.8	450	2900	3100	450	300	0.05	76	275	255	10	Grade T
Standard	Panel	P-021	UV Resistant	1.0	0.5	470	3000	3200	470	310	0.05	74	285	265	8	Grade U
Standard	Panel	P-022	UV Resistant	1.5	1.0	490	3100	3300	490	320	0.05	72	295	275	10	Grade V
Standard	Panel	P-023	UV Resistant	1.2	0.8	510	3200	3400	510	330	0.05	70	305	285	10	Grade W
Standard	Panel	P-024	UV Resistant	1.0	0.5	530	3300	3500	530	340	0.05	68	315	295	8	Grade X
Standard	Panel	P-025	UV Resistant	1.5	1.0	550	3400	3600	550	350	0.05	66	325	305	10	Grade Y
Standard	Panel	P-026	UV Resistant	1.2	0.8	570	3500	3700	570	360	0.05	64	335	315	10	Grade Z
Standard	Panel	P-027	UV Resistant	1.0	0.5	590	3600	3800	590	370	0.05	62	345	325	8	Grade AA
Standard	Panel	P-028	UV Resistant	1.5	1.0	610	3700	3900	610	380	0.05	60	355	335	10	Grade BB
Standard	Panel	P-029	UV Resistant	1.2	0.8	630	3800	4000	630	390	0.05	58	365	345	10	Grade CC
Standard	Panel	P-030	UV Resistant	1.0	0.5	650	3900	4100	650	400	0.05	56	375	355	8	Grade DD
Standard	Panel	P-031	UV Resistant	1.5	1.0	670	4000	4200	670	410	0.05	54	385	365	10	Grade EE
Standard	Panel	P-032	UV Resistant	1.2	0.8	690	4100	4300	690	420	0.05	52	395	375	10	Grade FF
Standard	Panel	P-033	UV Resistant	1.0	0.5	710	4200	4400	710	430	0.05	50	405	385	8	Grade GG
Standard	Panel	P-034	UV Resistant	1.5	1.0	730	4300	4500	730	440	0.05	48	415	395	10	Grade HH
Standard	Panel	P-035	UV Resistant	1.2	0.8	750	4400	4600	750	450	0.05	46	425	405	10	Grade II
Standard	Panel	P-036	UV Resistant	1.0	0.5	770	4500	4700	770	460	0.05	44	435	415	8	Grade JJ
Standard	Panel	P-037	UV Resistant	1.5	1.0	790	4600	4800	790	470	0.05	42	445	425	10	Grade KK
Standard	Panel	P-038	UV Resistant	1.2	0.8	810	4700	4900	810	480	0.05	40	455	435	10	Grade LL
Standard	Panel	P-039	UV Resistant	1.0	0.5	830	4800	5000	830	490	0.05	38	465	445	8	Grade MM
Standard	Panel	P-040	UV Resistant	1.5	1.0	850	4900	5100	850	500	0.05	36	475	455	10	Grade NN
Standard	Panel	P-041	UV Resistant	1.2	0.8	870	5000	5200	870	510	0.05	34	485	465	10	Grade OO
Standard	Panel	P-042	UV Resistant	1.0	0.5	890	5100	5300	890	520	0.05	32	495	475	8	Grade PP
Standard	Panel	P-043	UV Resistant	1.5	1.0	910	5200	5400	910	530	0.05	30	505	485	10	Grade QQ
Standard	Panel	P-044	UV Resistant	1.2	0.8	930	5300	5500	930	540	0.05	28	515	495	10	Grade RR
Standard	Panel	P-045	UV Resistant	1.0	0.5	950	5400	5600	950	550	0.05	26	525	505	8	Grade SS
Standard	Panel	P-046	UV Resistant	1.5	1.0	970	5500	5700	970	560	0.05	24	535	515	10	Grade TT
Standard	Panel	P-047	UV Resistant	1.2	0.8	990	5600	5800	990	570	0.05	22	545	525	10	Grade UU
Standard	Panel	P-048	UV Resistant	1.0	0.5	1010	5700	5900	1010	580	0.05	20	555	535	8	Grade VV
Standard	Panel	P-049	UV Resistant	1.5	1.0	1030	5800	6000	1030	590	0.05	18	565	545	10	Grade WW
Standard	Panel	P-050	UV Resistant	1.2	0.8	1050	5900	6100	1050	600	0.05	16	575	555	10	Grade XX
Standard	Panel	P-051	UV Resistant	1.0	0.5	1070	6000	6200	1070	610	0.05	14	585	565	8	Grade YY
Standard	Panel	P-052	UV Resistant	1.5	1.0	1090	6100	6300	1090	620	0.05	12	595	575	10	Grade ZZ
Standard	Panel	P-053	UV Resistant	1.2	0.8	1110	6200	6400	1110	630	0.05	10	605	585	8	Grade AA
Standard	Panel	P-054	UV Resistant	1.0	0.5	1130	6300	6500	1130	640	0.05	8	615	595	10	Grade BB
Standard	Panel	P-055	UV Resistant	1.5	1.0	1150	6400	6600	1150	650	0.05	6	625	605	10	Grade CC
Standard	Panel	P-056	UV Resistant	1.2	0.8	1170	6500	6700	1170	660	0.05	4	635	615	8	Grade DD
Standard	Panel	P-057	UV Resistant	1.0	0.5	1190	6600	6800	1190	670	0.05	2	645	625	10	Grade EE
Standard	Panel	P-058	UV Resistant	1.5	1.0	1210	6700	6900	1210	680	0.05	0	655	635	10	Grade FF
Standard	Panel	P-059	UV Resistant	1.2	0.8	1230	6800	7000	1230	690	0.05	-2	665	645	8	Grade GG
Standard	Panel	P-060	UV Resistant	1.0	0.5	1250	6900	7100	1250	700	0.05	-4	675	655	10	Grade HH
Standard	Panel	P-061	UV Resistant	1.5	1.0	1270	7000	7200	1270	710	0.05	-6	685	665	10	Grade KK
Standard	Panel	P-062	UV Resistant	1.2	0.8	1290	7100	7300	1290	720	0.05	-8	695	675	8	Grade LL
Standard	Panel	P-063	UV Resistant	1.0	0.5	1310	7200	7400	1310	730	0.05	-10	705	685	10	Grade MM
Standard	Panel	P-064	UV Resistant	1.5	1.0	1330	7300	7500	1330	740	0.05	-12	715	695	10	Grade NN
Standard	Panel	P-065	UV Resistant	1.2	0.8	1350	7400	7600	1350	750	0.05	-14	725	705	8	Grade OO
Standard	Panel	P-066	UV Resistant	1.0	0.5	1370	7500	7700	1370	760	0.05	-16	735	715	10	Grade PP
Standard	Panel	P-067	UV Resistant	1.5	1.0	1390	7600	7800	1390	770	0.05	-18	745	725	10	Grade QQ
Standard	Panel	P-068	UV Resistant	1.2	0.8	1410	7700	7900	1410	780	0.05	-20	755	735	8	Grade RR
Standard	Panel	P-069	UV Resistant	1.0	0.5	1430	7800	8000	1430	790	0.05	-22	765	745	10	Grade SS
Standard	Panel	P-070	UV Resistant	1.5	1.0	1450	7900	8100	1450	800	0.05	-24	775	755	10	Grade TT
Standard	Panel	P-071	UV Resistant	1.2	0.8	1470	8000	8200	1470	810	0.05	-26	785	765	8	Grade UU
Standard	Panel	P-072	UV Resistant	1.0	0.5	1490	8100	8300	1490	820	0.05	-28	795	775	10	Grade VV
Standard	Panel	P-073	UV Resistant	1.5	1.0	1510	8200	8400	1510	830	0.05	-30	805	785	10	Grade WW
Standard	Panel	P-074	UV Resistant	1.2	0.8	1530	8300	8500	1530	840	0.05	-32	815	795	8	Grade XX
Standard	Panel	P-075	UV Resistant	1.0	0.5	1550	8400	8600	1550	850	0.05	-34	825	805	10	Grade YY
Standard	Panel	P-076	UV Resistant	1.5	1.0	1570	8500	8700	1570	860	0.05	-36	835	815	10	Grade ZZ
Standard	Panel	P-077	UV Resistant	1.2	0.8	1590	8600	8800	1590	870	0.05	-38	845	825	8	Grade AA
Standard	Panel	P-078	UV Resistant	1.0	0.5	1610	8700	8900	1610	880	0.05	-40	855	835	10	Grade BB
Standard	Panel	P-079	UV Resistant	1.5	1.0	1630	8800	9000	1630	890	0.05	-42	865	845	10	Grade CC
Standard	Panel	P-080	UV Resistant	1.2	0.8	1650	8900	9100	1650	900	0.05	-44	875	855	8	Grade DD
Standard	Panel	P-081	UV Resistant	1.0	0.5	1670	9000	9200	1670	910	0.05	-46	885	865	10	Grade EE
Standard	Panel	P-082	UV Resistant	1.5	1.0	1690	9100	9300	1690	920	0.05	-48	895	875	10	Grade FF
Standard	Panel	P-083	UV Resistant	1.2	0.8	1710	9200	9400	1710	930	0.05	-50	905	885	8	Grade GG
Standard	Panel	P-084	UV Resistant	1.0	0.5	1730	9300	9500	1730	940	0.05	-52	915	89		

Table 3
The association of measures of mental health with smoking and tobacco exposure.

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• **ANSWER** The following table summarizes the results.

the day. Along with increasing the city center, it will also broaden the area planned for future industry (see Fig. 7). The 1945 seismic contour map shows in the city as the toothed neck will completely submerge under the water. According to measurements of the 1945 bathymetry, Shadai, Meadie, and Shadie creeks were inundated by the 1945 Tidal. So, it can be assumed that areas height that were at least 44 m high at the eastern bay of Gwadar. The beach topography at Gwadar is greatly altered by the construction of embankments on both sides of the bay, which reduces the efficacy of a comparative analysis with the 1945

100

The effect of a $\Delta = \text{high}$ wave in Form is almost the same as that of Gradient, but it has the potential for more damage to the infrastructure along the beach line (Fig. 8 in the supplement). The waves up to 7 m

the same submerged under the waves (see Fig. 8 in the supplement). The submerged area also provides about the dominant groundwater discharge. It will block the major road that links Paseo to the rest of the country. The 10 m base height has the greatest inundation in the other city centers, including Valdés, Flores and party Real. In this scenario, infrastructure and human life will suffer greatly. Almost the entire town will be affected in the 1.5-m scenario. At the city center, single-story buildings will be approximately 7–10 m underwater. The 1945 tsunami (12-m) composite inundation analysis fits well with our results in Paseo, except there is a land elevation of 900 m at the northern part of the city – 3 m high, and 1.5 km long eastern barrier built in the northern part area. These barriers provide some coastal defense against the waves, consisting three different sections.

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Fig. 4. A comparison of the DEM (left) and a point cloud (right) of a model coastline. The topographic map shows the mapping structure as compared with the ground truth data to the surroundings. The bottom point cloud plot is the raw point elevation data (black, open) used to build the DEMs (colored) used for the DEM image plot.

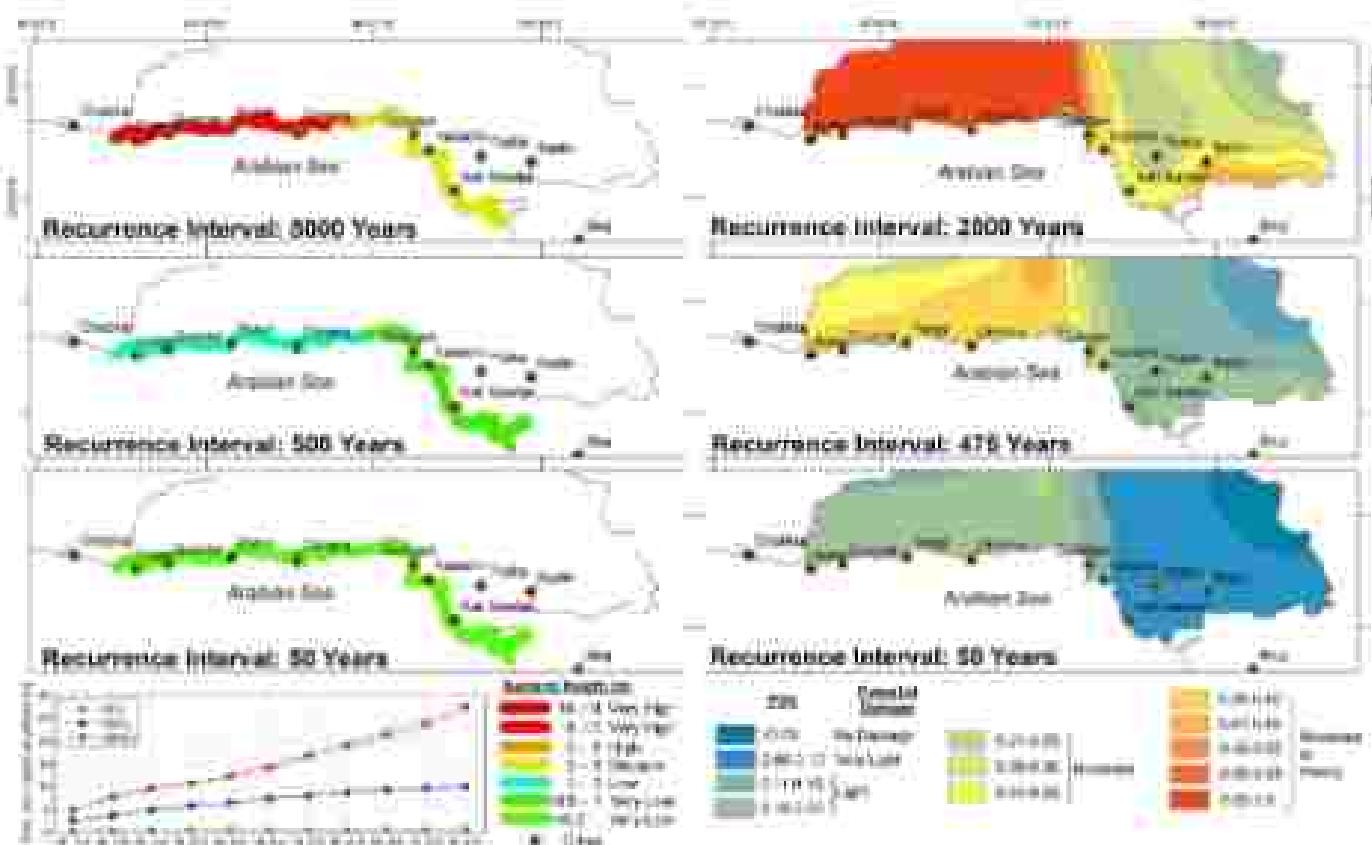


Fig. 5. The tsunamis return probability hazard height from different recurrence intervals by <https://doi.org/10.1016/j.jre.2020.103000>. The right column contains the return period-based hazard-based probability of given elevation (P_H) for different recurrence intervals by <https://doi.org/10.1016/j.jre.2020.103000>. The probability of inundated height's calculation was done according to the formulae shown at the right.

the eastern and western bays (see Fig. 9 in the supplement). In the 2-m scenario, the city undergoes minor damage to the houses along the coast, especially at the eastern bay. A 2-m tsunami wave has the potential to severely affect the town. As estimated from the literature of the 2004 tsunami, however, approximately a 7 m wave affected the town. In this regard, a 10 cm wave and a 15 cm wave have a potential to affect the whole town with 4 m and 9 m from depths respectively, also,

along with sea, the human built's rock (seabed) will completely submerge under the waves.

4.4. Durban

Durban is the biggest city along the Middle Coast and is highly vulnerable to the tsunami waves. Though the evaluated hazard generally

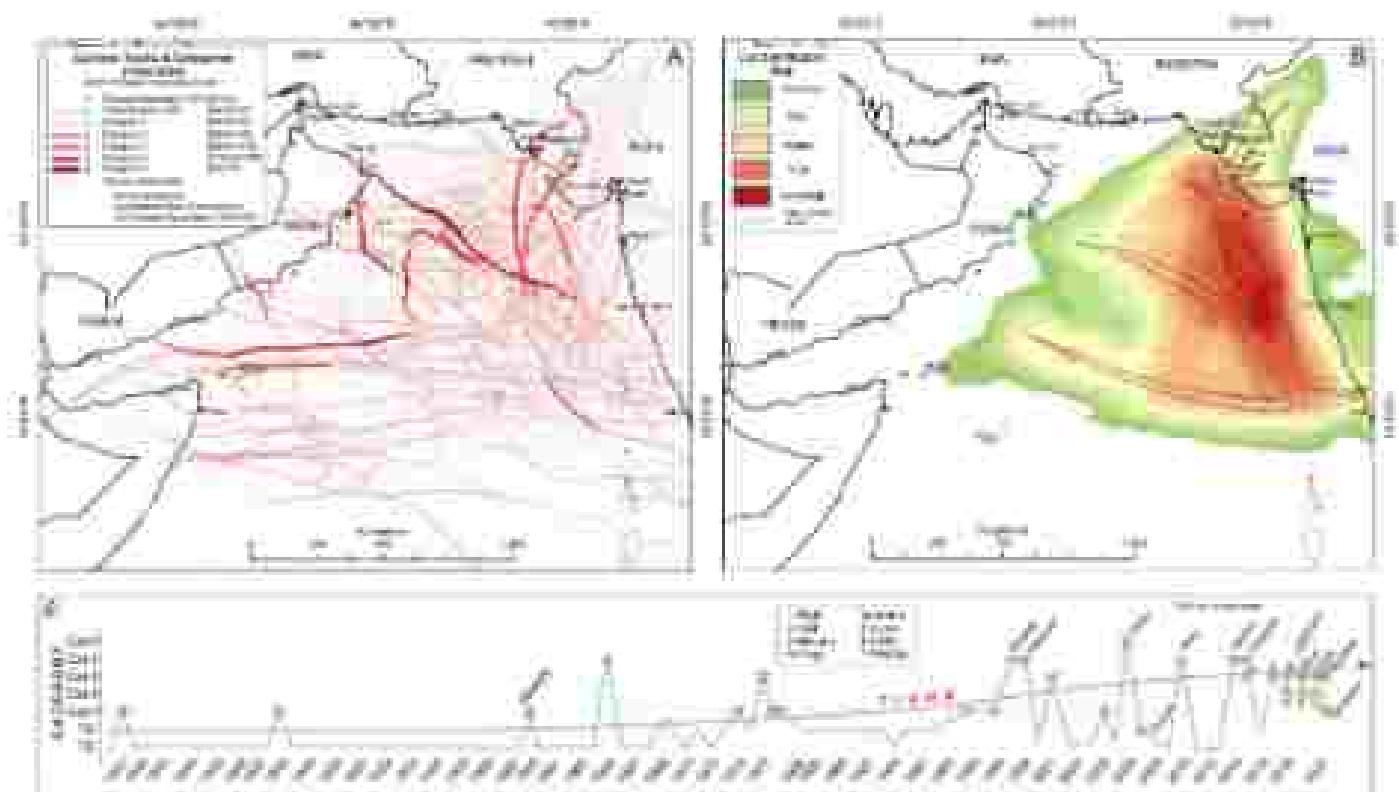


Fig. 2. A: Cyclic stress in the Alpine Piedmont 1840–1990 (International Sheet Tectonics Action in Central Europe (ISETACE), 2010). The expanded color scales show the seismic intensity. B: Cyclic terrain map based on the seismic activity. The colored structure shows the high frequency seismic trend. C: graph shows a trend in the frequency and intensity of quakes.

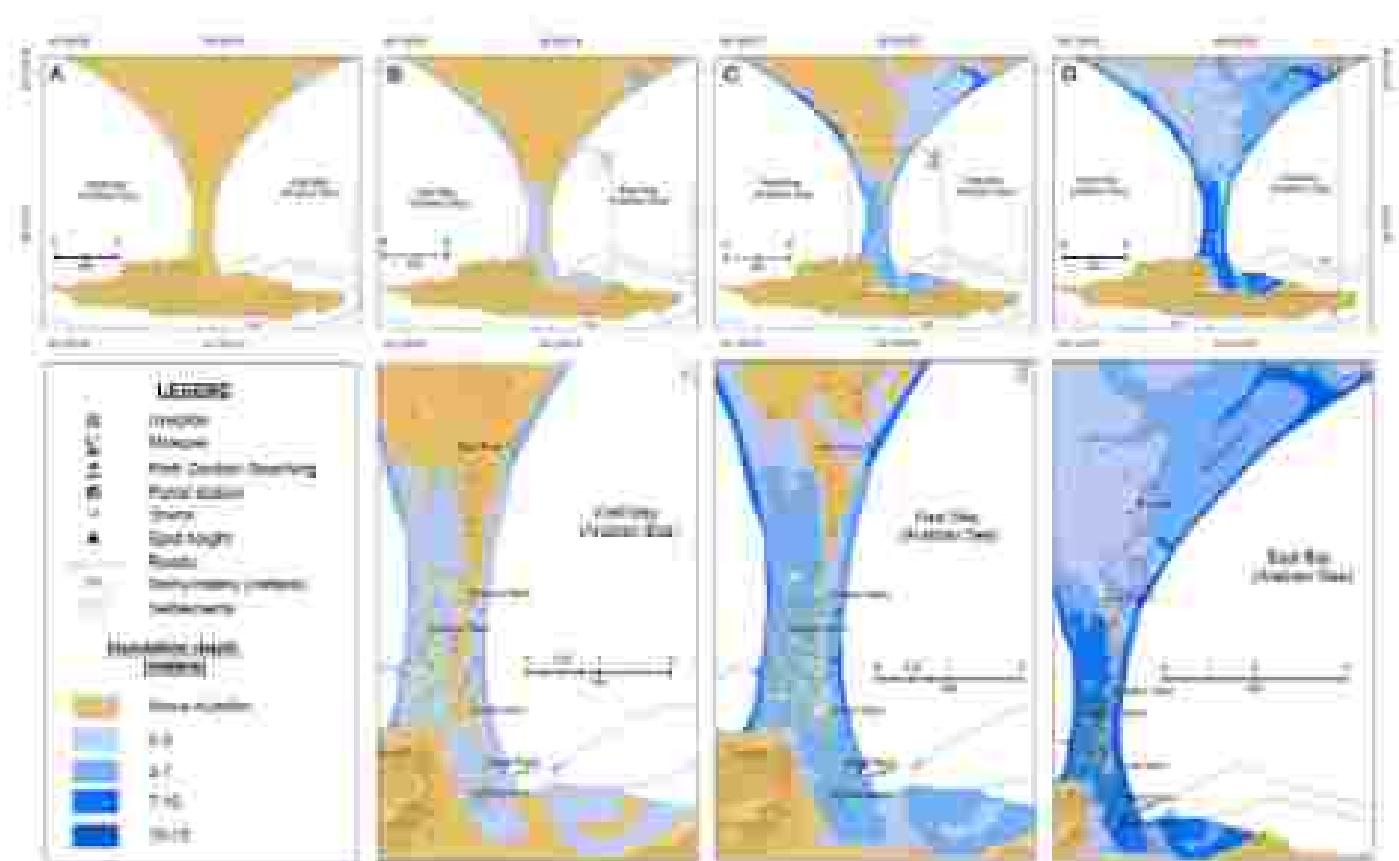


Fig. 3. Results of the seismic analysis for Croatia: A–D = seismic; E–H = seismotectonics.

the most extreme events is low enough for non-subsidence-based. It is known that a major impact on Japan, China, the Maluku Islands, and the British Holocene Authority (2021). The 7 m and 12 m + areas will have serious consequences due to their higher tendency to inundate deep into the city (see Fig. 13 in the supplement). Other than directly facing the shoreline, the cities (Lima, China, Cheng) provide safe pathways for a city and for inland inundation, therefore, prone to these critical situations (see Fig. 14). More of 15 h can cause significant loss of life and infrastructural damage. The Maluku and Fidji islands will be submerged for 10 h in the scenario. Over flooding, submerged buildings, proximity to the beach, and a poor economic situation increase the risk factor and reduce the society's resilience. Favelas has an advantage over the other cities in that it has many multi-story concrete buildings, which may prevent a quick initial inundation during such an event.

8. Discussion

The establishment of the maximum tsunami potential is essential for its hazard and risk assessment. In this regard, such approach has its own limitations, particularly gaps in knowledge, data and different computational types. As compared to a single point, the synergy of five groups has improved the level of information and confidence in the hazard estimation and risk assessment by complementary analysis for a scenario, the process from scientific to expert and citizens' data, narrow down the result's range. The probability studies based on various techniques and methods estimate the occurrence interval of a mega tsunami event between 1500 and 5000 years. These probabilistic models only take earthquakes occurs from account and do not consider other non-seismic seismic sources. In this regard, considering the well-studied historical events of 1643 CE and 1662 CE in the northern Andean Sea, the recurrence interval ranges from 1–1000 years. With these two arguments, a quick and crude analysis implies that non-seismic seismic sources and/or secondary sources (e.g., oceanic landslides) generate about 20%–30% of tsunami in the Andean Sea. This ratio is high, as we look the interpretation posterior. Globally, about 30% of tsunami have an anthropogenic source (Matsui et al., 2014; Lutjeh et al., 2017; Tolosa et al., 2021). Non-seismic tsunami sources, particularly underwater landslides, complies hazard assessment in terms of magnitude and occurrence interval. In comparison to an earthquake, a landslide has a greater potential for larger wave heights due to its large material size (see Table 28). However, the most triggered mass characterize the tsunami are. The 1643 major and 2013 minor events in the Andean Sea are also attributed to a submarine landslide triggered by offshore and terrestrial earthquakes, respectively. These recent tsunami events also set the high frequency of underwater slope failure (Fig. 13 in the supplement, Mader et al., 2018, 2020). Landslide stability is dependent on several geomorphological and biological factors (slope angle, consolidation, species growth, density, genetic type and sex, resistance value, and compaction). Any agents can lower the threshold of these factors, especially earthquakes, submarine volcanic activity, marine life, and submarine anthropogenic activities like communication cables, mining, and drilling. Considering according to high-magnitude earthquakes (Mw > 8), capable of developing a mega tsunami, because the frequency of large failures should be high because the = 1–2 years every 5 years (Table 3 in the supplement) and provides critical land slides that have complied to see the mitigation of their angle of repose (see Fig. 13 in the supplement). The landslides caused by and as an important research gap, which greatly affects the tsunami hazard assessment. The passive measurement and monitoring of these factors to estimate their risk and failure time is a mandatory task.

The compilation shows the sequence of events in a P-C, 1643 CE, 1662 CE, 1652 CE and 1662 CE in 1662 CE (see Fig. 13). The interval between the first three clusters of events is about 100 years, while the fourth and fifth clusters have an interval of 1100 years, but there is a gap of ~250 years between these two clusters of events. The large interval

(2000 years) has three possible explanations. First, there may be two clusters more areas and of the trend(s) occurred, either they could not be governed in the sedimentary record or, if preserved, they have not been discovered yet. This approach has limitations. 1) sedimentary records, which are highly susceptible to transgressive environments and have yet to be explored for paleotsunami. Deposits that could switch (e.g. Fig. 4) the status of interpretation for most of them as transgressive deposits and not proven yet, as their deposition by other non-seismic seismic sources and cycles is not completely ruled out. 2) wave approximation, geological information and assumptions, and the correlation of an event's name with a probably known event, such as the 1643 CE tsunami.

For Ezeiza, the inundation analysis and the results are quite simple to conclude. The probability is due to the city's tightly packed nature and the confusing human current were record. In the tightly packed area, MIL could not update the streets, roads, and buildings to some scale. The maximum wave height expected for this location is its location, as the high intensity waves generated at MIL, mostly propagating in a north-south direction, and the city lies mostly in the east of MIL. However, the deterministic model shows that Ezeiza expects about half of the maximum wave height recorded at the Compte, as is the case today, located in a completely different from MIL, and is the closest to Ezeiza of the three. If we apply this expectation to the 1643 tsunami waves, the maximum wave height calculated and estimated at Compte was about 10 m (Table 4, Fig. 14). The calculation is based on claims made by 1643 tsunami survivors that waves did not approach 1 m after the earthquake (Bragg, 1643; Bragg, 1643; Bragg, 1643), along with interviewing older citizens, provided solid evidence of 1 m height in Ezeiza, wave height was about 1.2 m as reported by the 1643 tsunami survivors (Table 4, Fig. 14), but according 1643, the gauge record shows a 22 cm amplitude (Bragg, 1643; Bragg, 1643). Taking this hypothesis ahead for the New 0.0 (25 m slip) earthquake at MIL, it could generate a maximum wave height of 15 m at Compte and 10 m at Ezeiza's Coast. The oceanic ground plane near Ezeiza is at least 15 m below the high tide line. Through the above discussion, we may conclude that this part of the study area is certainly to receive waves higher than 9 m. However, no vulnerability with reference to a mega-tsunami needs evaluation.

The Malvinas Coast, due to its proximity to the MIL could not benefit much from the far-field early warning system, namely the Indian Ocean Tsunami Warning and Mitigation System (OTWS) installed in the Andean Sea. With reference to a tsunami generated at MIL, the system is functional for the south-eastern Indian Ocean (including Sumatra, and its far-field warning mechanism; see main text), numerical modeling of 1643 transgressive catastrophe (Fig. 14) shows that the source measure of Peru, Chile and Deep-Ocean Assessment and Reporting of Tsunami (sums, 2017) occurs in about 15, 100 and 15 min respectively (Table 3 in the supplement). There is a reaction time of 3 min for alert issuance and evacuation. The Chilean Meteorological Department (CDD) is the final agency for monitoring coastal hazards and issuing warning alerts. CDD established a near-field tsunami early warning system in 2010 and a well-established mid-ocean early warning system in the 1990s. In the 1990s, the tsunami reaction time (15 min) of the nearest event is commonly short. On the resilience and preparedness side, the large number of casualties and property losses during the cyclone of 1999, 2004, 2007, 2013 and 2020 suggest a delayed level of preparedness and resilience and for tsunami the level is even more pathetic.

9. Conclusion

This study includes a hazard and risk assessment of transients and cyclones along the Malvinas Coast of Chilean. The highly populated coastal (~ 10 million), semi-industrial economy side of Compte and northern port cities along the coast have heightened the focus of the topic. We used five approaches including probabilistic, demographic, geographical, sedimentary and historical analysis to estimate the wave

geomorphic in the section 8m and constituted four different wave conditions for initiation and/or transience scenarios were performed in a 3D environment coupled with land use maps to determine the areas at high and very high risk.

Despite evidence for high risk of seismic, Tsunami and Gender due to their combination, low probability and proximity to the tsunami source. However, a 3 m elevation will not have a significant impact on the tsunami due to human's 4 m caused by newly constructed higher-ups on both sides (see Fig. 1). Karpinski is the most vulnerable site to extreme events due to population pressure, infrastructure buildings, proximity to the beach, and a poor economic situation that increase the risk factors and reduce the society's resilience. However, the question with respect to the RMT and propagating waves needs to be studied in detail. Furthermore, the nuclear power plant was Kurskii is at least 15 m above the high tide line and is therefore safe, provided no other components like the cooling system, keep on working during the extreme wave event (see Fig. 10 in the supplemental). Overall, the statistical analysis shows that the damage to the Khabarovsk Coast will be minor for 3 m, moderate for 7 m and severe for the 10–15 m and highest tsunami, especially for the three major sites discussed in the paper. In these four cities, at least 0.7 million people are at risk. Aside from human hazard, the intensity and frequency of seismic since 1955 have increased significantly. From Tropical Storm to Category-3 with two cyclones per year expected. The number of number of wave and tsunami events during the cyclone in 1990, 1999, 2000, 2001, and 2021 reflect a decreasing level of predictability and resilience, and the level is even more peaked in the case of tsunami.

3.3. Outlook

The coastline along the high tsunami hazard zone (Gender to Somar) should be strengthened for field evidence (measured) to better the paleo-tsunami record to estimate severe potential and the consequences thereof. The entire seismic zone south of Tsim is related to the Somar Fault, the nature of the decollement boundary and the deep GRS Valley at Tsim, needs to be investigated in detail. Seismic evidence potential is poorly known, but detailed bathymetric studies may help in determining its potential. Further, the high hazard zones determined in the study may be revisited at regular intervals to estimate the slip rate and time of mega failure. Though the forecasting side of cyclones has greatly improved with time, the resilience and predictability experts are still far from satisfactory. The coasts of Somar and Tsim are susceptible to the megavine formation. The Somar Beach (Cifford) may be turned into artificial spurs that may serve the purpose of planting a mangrove forest. Along with many other benefits, it may act as a coastal defense structure against the extreme waves. There is a considerable scientific understanding of the contiguous and human dynamics, but still there are large data and knowledge gaps, uncertainties and inconsistencies. Due to these shortcomings and limitations, the timing and exact location of catastrophic and tsunami are still unpredictable (Pöhl et al., 2021). Till there is a scientific breakthrough, the society is hard to be get well prepared by enhancing the community awareness and resilience.

Authors contributions

AP contributed the idea, planned the methodology, interpreted the results, and then drafted the document. SP assisted in data interpretation and writing process. GP recognized the process, AP represented the whole process and provided technical, conceptual, financial, and political support for the research work.

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Declaration of competing interest

The authors declare that they have no known competing financial interests or personal relationships that could be construed to influence the work reported in this paper.

The authors declare the following financial conflicts/personal relationships which may be considered as potential competing interests.

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Data availability

All relevant documents and newspaper extracts that the paper refers to can be found at <https://doi.org/10.4236/ojs.202111121>. The 10 m digital elevation model data is owned and processed by the German Aerospace Center.

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Appendix A. Supplementary data

Supplementary data to this article can be found online at <https://doi.org/10.4236/ojs.202111121>.

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